

This report is incomplete.

The original, printed version had missing pages. We apologize for the inconvenience.



VULCAN WELL EVALUATION SUMMARY

by: G.M. Gabert

May 1962

ANJ 8757

ALBERTA RESEARCH COUNCIL LIBRARY  
5TH FLOOR, TERRACE PLAZA  
4445 CALGARY TRAIL SOUTH  
EDMONTON, ALBERTA, CANADA  
T6H 5R7

25 p's

Vulcan Well Evaluation Summary

17 - 24 - W4

by

G.M. Gabert

May 29, 1962



MEMORANDUM

RESEARCH COUNCIL OF ALBERTA, UNIVERSITY OF ALBERTA, EDMONTON

Mr. D. H. Lennox

FROM P. Meyboom

DATE June 26, 1961.

Re: Water supply, Vulcan

Some comments on the letter of C. C. Parker, Whittaker & Co., dated June 9, 1961.

1. If the pump test data are plotted on semi-logarithmic paper and analyzed by means of the modified Theis non-equilibrium formula, the transmissibility of the aquifer appears to be 1020-1240 gpd/ft. which is considerably higher than the transmissibility of the Paskapoo formation within the town limits (170 gpd/ft.). The drawdown graph shows a characteristic succession of steps, each of which suggests stabilization of pumping level. Six steps of apparent stabilization can be recognized in the pump test, varying in duration from 30 minutes to 3 hours. The least apparent stabilization has obviously been interpreted as a true equilibrium between recharge and discharge, but the remainder of the drawdown graph does not justify this assumption, and consequently, the calculation of specific capacity has no validity.

The periods of apparent stabilization during the pumping test are probably not related to aquifer conditions, but rather to variations in pumping rate. According to the information that is submitted by the engineering firm, the average pumping rate is 99 gpm. However, the meter-readings indicate a pumping rate of 80 gpm. from 11:45 a.m. - 1:35 p.m. on February 1, whereas the average pumping rate from 1:35 p.m. to 11:05 p.m. of the same day amounts to 106 gpm. Judging from the drawdown graph the pumping rate of 80 gpm. is valid for 200 minutes. followed by a pumping rate of 120 gpm. per minute from 200 - 500 minutes. Pumping tests with variations in discharge exceeding 1 - 5% have no value for hydrologic interpretation as the assumptions of the mathematical model are no longer valid.

If it were to be assumed that the pumping test had some value, the safe pumping rate from this well varies between 40 and 50 gpm., which has been calculated by the conventional safe yield computation:

$$Q = \frac{\text{Transmissibility} \times \text{available drawdown}}{2110} \times 70\%$$

in which: T = 1020 gpd/ft - 1240 gpd/ft

drawdown = 120 feet.

Summarizing, it may be stated that the variations in pumping rate render it impossible to interpret correctly the results of the pumping test. If pumping test data were assumed to be reliable, the safe yield of this aquifer would be 50 gpm at the most, but probably less. The estimate submitted by the engineering firm is based on the wrong assumption that the principle of specific capacity can be applied to a non-equilibrium pumping test, and as a result their estimate of safe yield is nearly five times too high.

## Contents

	Page
Introduction	1
Step - Drawdown Test Results	1
Constant - Rate Tests Results	1
Table I	2
Table II	2
Table III	5
Table IV	5
Table V	6
Water Quality	6
Conclusions and Recommendations	6
Future Prospects for a Groundwater Supply	7
Appendix A	
Appendix B	

## VULCAN WELL EVALUATION SUMMARY

### Introduction

Constant-rate and step-drawdown tests were conducted on 5 wells at the Town of Vulcan, Alberta, during the period from May 8 to May 13, inclusive, to evaluate the potential long range production capacity of the wells.

A step-drawdown test was conducted on each well to establish its efficiency at various pumping rates and evaluate the well design.

A constant-rate test was performed on well #3 in order to calculate the aquifer coefficients which are necessary to estimate future pumping levels for extended periods of continuous pumping.

### Step-Drawdown Test Results

The step-drawdown tests indicate that the five wells are not efficient or designed for pumping rates exceeding 7 gpm when each well is pumped separately. At pumping rates lower than 7 gpm the wells function satisfactory.

### Constant-Rate Tests Results

The average transmissibility of the water bearing materials was calculated from the time-drawdown data obtained from this test. The average transmissibility determined was 246 gpd/ft., which means the aquifer yields its water slowly to a well.

Observations of drawdowns in wells number 1, 2, 4, and 5 during the constant-rate and step-drawdown tests revealed that interference occurs between wells number 1, 2, 3, and 4. Well No. 5 was not influenced by the pumping of the other four wells.

Table I indicates that the predicted pumping levels in the aquifer under conditions of continuous pumping at a constant rate conform rather closely to the actual drawdowns which occur for a given pumping period.

**Table I:** A comparison of estimated drawdowns in the aquifer and actual drawdowns measured at the end of 24 hours of continuous pumping at a rate of 5.2 gpm.

Well No.	Status	Estimated Drawdowns	Actual Drawdowns
2	Observation	44.5'	46.0'
3	Pumping	7.5'	7.8'
4	Observation	5.1'	3.9'

During the pumping test the top of the lower aquifer was considered to be 142 feet. The maximum safe pumping rate is that rate which will not draw the water below the top of the lowest water bearing zone over an extended period of continuous pumping at a constant rate. For estimating future pumping rates 90 feet of available drawdown was considered a maximum in the calculations.

**Table II:** Estimate future pumping levels in the aquifer at various distances from the center of a pumping well.

Radial Distance from center of pumping well (ft.)	Continuous Constant pumping Rate (gpm)	Length of Pumping Period	Estimated Drawdowns in Aquifer (ft.)
0.25	5.2	1 day	44.5
10	5.2	1 day	26.6
100	5.2	1 day	15.5
1000	5.2	1 day	4.6
0.25	5.2	30 days	49.5
10	5.2	30 days	34.8
100	5.2	30 days	23.7
1000	5.2	30 days	12.6
0.25	5.2	90 days	55.4
10	5.2	90 days	37.5
100	5.2	90 days	26.4
1000	5.2	90 days	15.4

Table II (cont'd)

Radial Distance from center of pumping well (ft.)	Continuous Constant pumping Rate (gpm)	Length of pumping period	Estimated Drawdowns in Aquifer (ft.)
0.25	5.2	5 years	62.7
10	5.2	5 years	45.0
100	5.2	5 years	33.9
1000	5.2	5 years	22.5
0.25	6	1 day	51.5
10	6	1 day	30.8
100	6	1 day	17.9
1000	6	1 day	5.3
0.25	6	30 days	57.1
10	6	30 days	40.3
100	6	30 days	27.3
1000	6	30 days	14.6
0.25	6	90 days	64.1
10	6	90 days	43.4
100	6	90 days	30.5
1000	6	90 days	17.6
0.25	6	5 years	72.5
10	6	5 years	52.0
100	6	5 years	39.0
1000	6	5 years	26.0
0.25	7	1 day	60.0
10	7	1 day	35.8
100	7	1 day	20.9
1000	7	1 day	6.2
0.25	7	30 days	66.5
10	7	30 days	47.0
100	7	30 days	31.9
1000	7	30 days	17.0
0.25	7	90 days	74.6
10	7	90 days	50.5
100	7	90 days	35.5
1000	7	90 days	20.5

Table II (cont'd)

Radial Distance from center of pumping well (ft.)	Continuous Constant pumping Rate (gpm)	Length of pumping period	Estimated Drawdowns in Aquifer (ft.)
0.25	7	5 years	84.5
10	7	5 years	60.6
100	7	5 years	45.6
1000	7	5 years	30.3
0.25	10	1 day	85.7
10	10	1 day	51.2
100	10	1 day	29.8
1000	10	1 day	24.5
0.25	10	30 days	95.0
10	10	30 days	67.0
100	10	30 days	45.6
1000	10	30 days	24.2
0.25	10	90 days	106.6
10	10	90 days	72.2
100	10	90 days	50.8
1000	10	90 days	29.3
0.25	10	5 years	120.6
10	10	5 years	86.6
100	10	5 years	65.2
1000	10	5 years	46.6

Table III: 24 hour interference due to the pumping of each well at various constant rates

Pumping Well No.	Well influenced by pumping	Radial Distance from pumping well (ft.)	Interference in feet of drawdown at various pumping rates			
			Pumping rate gpm	5.2	6	7
1	2	414		8.7	10.1	11.7
	3	1300 *		3.6	4.2	4.9
	4	1800 *		2.2	2.5	2.9
2	1	414		8.7	10.1	11.7
	3	930		5.1	5.9	6.8
	4	1400 *		3.1	3.6	4.2
3	1	1300 *		3.6	4.2	4.9
	2	930		5.1	5.9	6.8
	4	523		7.5	8.7	10.1
4	1	1800 *		2.2	2.5	2.9
	2	1400 *		3.1	3.6	4.2
	3	523		7.5	8.7	10.1

\* Estimated Distances

Table IV: 90 day interference due to the pumping of each well at various constant rates

Pumping Well No.	Well influenced by pumping	Radial Distance from pumping well (ft.)	Interference in feet of drawdown at various pumping rates			
			Pumping rate gpm	5.2	6	7
1	2	414		19.6	22.7	26.4
	3	1300 *		14.0	16.2	18.9
	4	1800 *		12.6	14.6	17.0
2	1	414		19.6	22.7	26.4
	3	930		15.7	18.2	23.7
	4	1400 *		13.6	15.7	18.2
3	1	1300 *		14.0	16.2	18.9
	2	930		15.7	18.2	23.7
	4	523		18.4	21.3	24.8
4	1	1800 *		12.6	14.6	17.0
	2	1400 *		13.6	15.7	18.2
	3	523		18.4	21.3	24.8

Table V: Total interference in each well with all wells pumping at 5.2 gpm for a period of 1 day and 90 days.

Well No.	Pumping Period	Total Interference in feet
1	1 day	14.5
2	1 day	16.9
3	1 day	16.2
4	1 day	12.8
1	90 days	46.2
2	90 days	48.9
3	90 days	48.1
4	90 days	44.6

### Water Quality

The chemical quality of the water is acceptable in all the wells except well No. 5. This water in well No. 5 is not acceptable for a public supply due to the nitrate content. Pumping this well for several days may eliminate the nitrates or reduce their concentration to an amount not dangerous for public use.

The pumping of well No. 3 for 27 hours improved its chemical quality, particularly by reducing the sulphate concentration.

### Conclusions and Recommendations

1. Continuous pumping of a well in this aquifer at 5.2 gpm for a period of 90 days causes a drawdown of 55.4 feet (Table II) in the aquifer at the well perimeter. If wells 1, 2, 3, and 4 are pumped continuously at 5.2 gpm for a 90 day period, there will be an additional drawdown of nearly 50 feet (Table 5) in each well due to interference. This gives a total drawdown in each well that exceeds the total available drawdown of 90 feet.

Therefore, wells 1, 2, 3, and 4 cannot be pumped continuously at a constant rate of 5.2 gpm for a 90 day period.

2. Table II indicates that if each well was separately pumped continuously at 7 gpm for a 90 day period, the drawdown in each well would be very close to the maximum drawdown available. Thus if 2, 3, or 4 wells were pumped and were influenced by one another to the extent that these 4 wells are, each well should not be pumped over 5 gpm, a maximum rate for extended periods of continuous pumping.

3. Well No. 5 was not influenced by the pumping of wells No. 1, 2, 3, and 4. However, for a 90 day continuous pumping period the maximum rate at which well no. 5 can be pumped is 7 gpm.

#### Future Prospects for a Groundwater Supply

The Town of Vulcan should be aware that the prospects for drilling high capacity wells with an acceptable water quality are not good in the vicinity of the town. The recent pumping test indicates that wells yielding water from the same aquifer will have to be spaced at least 1000 feet apart, instead of the previously suggested 500 feet, in order to ensure minimum interference. Closely spaced wells will have to be pumped at low rates and thus a well field will be required to obtain a large quantity of water over a long period of time.

Unfortunately, drillers' logs for test holes and well previously drilled at Vulcan do not give reliable information on the thickness, depth, and availability of the various water bearing formations encountered. A future test drilling program should be carefully planned and properly supervised during its initial stage to ensure that each hole is properly sampled in order to obtain a detailed log of the formations encountered. In addition, an electric log may be run on several test holes to aid in establishing the exact position of waterbearing formations encountered during drilling. A competent water well driller experienced in this type of test drilling is a necessity for such a program.

Detailed, reliable information obtained from a well planned, properly executed test drilling program may result in improved well design and completion, and a more satisfactory performance from a single well or group of wells.

G. M. Gabert  
Groundwater Division  
Research Council of Alberta  
May 29, 1962.

**Appendix A**

*C. C. Parker, Whittaker & Company Ltd.*

R E P O R T

o n

INVESTIGATION FOR EXTENSION OF WATER SUPPLY

TOWN OF VULCAN

SCOPE OF WORK

The existing water supply for the Town of Vulcan, with a population of 1340 persons, comes from five or six wells located in the vicinity of the townsite. The water is pumped from the wells to an elevated storage tank (50,000 gallons) and thence, by gravity, to the distribution system.

The wells provide an aggregate flow of 65 US GPM which does not meet the present demand. Several of the larger users, the hotel, hospital, etc., have individual supply.

The quality of the water is unsatisfactory, having a high concentration of objectionable salts, nitrates and sulphates (Glauber Salts).

It has been necessary to investigate, test and analyze all the available sources of water in the district, to obtain an adequate source of potable water within the economic range of the community.

**BASIC DESIGN**

We established the necessary criteria for the study from sound engineering principles in accord with Health Regulations, and other basic requirements, as follows :-

1. Population Trend

Vulcan is the centre of a trading area with a radius of 30 to 35 miles. It is the County seat for the County of Vulcan.

There are no large industrial plants in the area, but some expansion may come from the presence of producing gas wells and their subsidiary extraction processing.

The estimated population for 1985, based on a plotted curve for the increase of the past fifteen years, is 3500. This figure has been used for our calculations throughout this report.

2. Quantity of Water

Water demand is estimated to be in U.S. Gallons.

<u>1961</u>		<u>1985</u>
233,000 gal.	Maximum daily demand	787,500 gals.
357,600 gal.	Fire demand	<u>561,200 gals.</u>
<u>590,600 gal.</u>	TOTAL	1,348,700 gals.

3. Storage

The reserve storage required is calculated to be 600,000 U.S. Gallons. This computation takes into account the existing 50,000 gallon elevated storage and a continuous pumping capacity of 250 gpm.

Supply line sizing has been determined, based on 250 U.S. gpm against a head of 160 feet and the 6" dia. main will suffice.

#### 4. Quality of Water

Chemical analyses of water from the local wells, surface water sources etc., indicate that there are several problems in the provision of a potable supply. These are :-

1. Reduction of Total Solids content.
2. Reduction of Sulphate content.
3. Reduction of Iron Manganese content.      2-2

Various methods of treatment have been studied, individually or in combination, to arrive at a satisfactory supply within the economic range for initial and operational costs.

The most efficient treatment that is within a reasonable cost range, is a combination filter and aerating plant. The filter selected is designed to remove iron and magnesium and will, in this process, combined with aeration, reduce the quantity of salts to a reasonable maximum.

### PHYSICAL STUDY

#### SOURCES OF WATER

##### a) Town Wells

A complete study of the possible wells in the vicinity of the townsite was made, using information obtained locally and from the Research Council of Alberta. The flow from wells in the area is small and it would require a large number of wells in series to meet the demand.

The quality of the water obtained from the existing wells varied widely, and treatment would be necessary as mentioned.

*C. C. Parker, Whittaker & Company Ltd.*

b) Airport Wells

The Town of Vulcan recently acquired the use of several wells that formed the source of supply for the RCAF Station, about 4.75 miles from the townsite.

A pump test on Well #1 was made early in February last and computations from the observations, indicate that these wells will produce 160 US gpm, which, combined with the production of wells at the airport and the town wells, and the proposed storage reservoir, will supply town needs adequately.

The quality of the water is comparable to that discussed and treatment will be required to reduce the total solids, sulphates and iron content.

c) Surface Water - Snake Creek

There is a potential source of surface water in the Snake Creek drainage basin. This would have to be developed in conjunction with the P.F.R.A. who would provide the impounding reservoir.

The length of the supply line would exceed that from the Airport by 2.25 miles approximately, and treatment would be the same problem due to salts in the ground water.

The cost analysis of this proposed source has been compared with the cost of the other installations.

The selection of a source of water hinges on the economics of the system to be proposed as there is no apparent advantage in quality of the product of either source, and sufficient water is available at either (b) or (c).

D E S I G N

SOURCE - Airport Wells - SCHEME B

- |                         |         |
|-------------------------|---------|
| 1. Use - Well #1 (RCAF) | 160 gpm |
| Well #2 (RCAF)          | 40 gpm  |

Use additional wells from townsite or airport as required. These wells should be pumped periodically to maintain quality (2 or 3 times weekly)

50 gpm  
250 gpm

2. Repairs to Wells at Airport.

Well #1 - Casing is corroded and rust particles are evident in discharge. The casing should be replaced and the well sealed off at 30 foot level.

Well #2 - This well to be flushed and gravel packed to overcome turbidity now prevalent.

3. Pipe Line - 6" diameter main to treatment plant and reserve storage. Pipe salvaged from existing line at airport satisfactory and less costly.
4. Pump to elevated storage from treatment plant and reserve.

A direct by-pass from reserve storage to the system for fire flow is feasible.

SOURCE - Snake Creek - SCHEME C

1. Impounding Reservoir - To be built in conjunction with or by P.F.R.A.
2. Intake Structure - normal for lake.
3. Pumping from reservoir to treatment plant and reserve storage - Supply line 7 miles long approximately.
4. Treatment etc., to distribution system as in Scheme B.

*C. C. Parker, Whittaker & Company Ltd.*

**COST ESTIMATES**

**B. AIRPORT SUPPLY**

	<u>Total Cost</u>	<u>Cost to Town Under Winter Works</u>
1. Pipe Salvage	\$35,210.91	\$23,252.07
2. Supply Line	38,194.58	26,577.74
3. Treatment Plant & Equipment	33,689.00	30,320.00
4. Reservoir	<u>38,146.00</u>	<u>34,330.00</u>
Sub-Total	145,240.49	114,479.81
10% Contingencies	<u>14,524.51</u>	<u>14,524.51</u>
	159,765.00	129,004.32
Engineering 6%	<u>9,585.00</u>	<u>9,585.00</u>
TOTAL	\$169,350.00	\$138,589.32

**C. SNAKE CREEK PROJECT**

Total Cost Pipeline	177,036.00
Storage and treatment	<u>93,923.00</u>
	\$270,959.00

NOTE:- This does not include the cost of the impending dam or property acquisition.



May 16, 1962

WATER ANALYSIS REPORT  
 CHEMICAL

Submitted by Secretary-Treasurer Date received May 14, 1962  
 Address Town of Vulcan Date reported \_\_\_\_\_  
Vulcan, Alberta Source of Sample Vulcan Well # 1  
 Container No. D.1 Serial No. \_\_\_\_\_  
 Lab. No. 62 - 4370

PARTS PER MILLION

Component	Value	Value	Value
Total Solids	858		
Ignition Loss	98	00	
Hardness	50	245	
Sulphates	245	17	
Chlorides	17	308	
Alkalinity	305	100	
Nature of Alkalinity	Bicarbonate of soda, lime	10.79011	
Nitrites	trace	1.00000	
Nitrates	1.2	0.10000	
Iron	0.2	0.47040	
Fluorine		0.10000	
REMARKS:		0.00000	
	Soda - 18.9 grains/gallon.	0.74000	
	plants. Water is chemical	47.00000	
		0.07010	

*Not based on  
 Sotted from ~~to the well~~  
 Water struck at ~~115.5~~  
 55:145-15*

e Aluminum and hard

C. Emerson Noble  
 Provincial Analyst

CEN:pl  
 cc Research Council



May 16, 1962

**WATER ANALYSIS REPORT  
 CHEMICAL**

Submitted by Secretary-Treasurer Date received May 14, 1962  
 Address Town of Vulcan Date reported \_\_\_\_\_  
Vulcan, Alberta Source of Sample Vulcan Well # 2  
 Container No. A 6 Serial No. \_\_\_\_\_  
 Lab. No. 62 - 4371

PARTS PER MILLION

*Slotted 60 to 150 ft  
 Water at 68 & 145 to 150 ft*

Total Solids	912		
Ignition Loss	50		
Hardness	50		
Sulphates	312		50 S
Chlorides	26		312 S
Alkalinity	275		26 C
Nature of Alkalinity	Bicarbonate of soda, lime and magnesium		275 C
Nitrites	trace		0.6 C
Nitrates	0.6		14.07605 S
Iron	0.3		13.07605 S
Fluorine			1.00000 S

REMARKS:

Soda - 16.7 grains/gallon. Soda may corrode Al plants. Water is chemically suitable.

22.02000 S  
 7.10400 S  
 55.41340 S  
 5.00000 S  
 30.07300 S  
 0.30440 S

C. Emerson Noble  
 Provincial Analyst

GEN:pl  
 cc Research Council



May 16, 1962

WATER ANALYSIS REPORT  
 CHEMICAL

Submitted by ..... Secretary - Treasurer ..... Date received ..... May 14, 1962 .....  
 Address ..... Town of Vulcan ..... Date reported .....  
 ..... Vulcan, Alberta ..... Source of Sample ..... Vulcan, Well #3, Sample # .....  
 Container No. .... 352 ..... Serial No. .... *At beginning of pumping* .....  
 Lab. No. .... 62 - 4367 .....

PARTS PER MILLION

Total Solids	1064		
Ignition Loss	56		
Hardness	50		50 S
Sulphates	404		404 S
Chlorides	19		19 S
Alkalinity	270		270 S
Nature of Alkalinity	Bicarbonate of soda, lime and magnesium		0 S
Nitrites	nil		16.03580 Al
Nitrates	nil		10.03500 Ca
Iron	nil		1.00000 Mg
Fluorine			10.10000 Cl
			0.00000 SO <sub>4</sub>
			9.40000 NO <sub>3</sub>
			0.00000 Fe

*Slotted 70 to 180 ft  
 Water at 80 + 145 to 150 ft*

REMARKS:

Soda - 16.3 grains/gallon. Soda may corrode Alu plants. Water is chemically suitable.

*CEN*  
 C. Emerson Noble  
 Provincial Analyst

CEN:pl  
 cc Research Council



May 16, 1962

**WATER ANALYSIS REPORT  
 CHEMICAL**

Submitted by Secretary-Treasurer Date received May 14, 1962  
 Address Town of Vulcan Date reported \_\_\_\_\_  
Vulcan, Alberta Source of Sample Vulcan Cell # 3 Dept #  
 Container No. D 29 Serial No. After 24 hours of purification  
 Lab. No. 62 - 4372 5.2 g/l

**PARTS PER MILLION**

Total Solids	824	50	
Ignition Loss	72	253	
Hardness	50	26	
Sulphates	253	245	
Chlorides	26	0	
Alkalinity	245	11.93800	11
Nature of Alkalinity	Bicarbonate of soda, lime and magnesium	10.95800	11
Nitrites	nil	1.00000	11
Nitrates	nil	6.32500	11
Iron	0.2	0.73000	11
Fluorine		4.90000	11
		0.00000	11
		21.05000	11
		0.30000	11
		32.00000	11
		0.10100	11
		40.00000	11
		0.00000	11

**REMARKS:**

Soda - 14.5 grains/gallon. Water is chemically suitable.

*C. Emerson Noble*  
 C. Emerson Noble  
 Provincial Analyst

CEN:pl  
 cc Research Council

C. EMERSON NOBLE  
 CHEMICAL ENGINEER  
 DIRECTOR INDUSTRIAL LABORATORIES  
 PROVINCIAL ANALYST



EDMONTON, ALBERTA  
 CANADA

May 16, 1962

WATER ANALYSIS REPORT  
 CHEMICAL

Submitted by Secretary Treasurer Date received May 14, 1962  
 Address Town of Vulcan Date reported \_\_\_\_\_  
Vulcan, Alberta Source of Sample Vulcan Well # 4  
 Container No. 225 Serial No. \_\_\_\_\_  
 Lab. No. 62 - 4358

PARTS PER MILLION

Total Solids	930	
Ignition Loss	36	
Hardness	50	
Sulphates	362	50
Chlorides	15	362
Alkalinity	240	15
Nature of Alkalinity	Bicarbonate of soda, lime and magnesium	240
Nitrites	trace	0
Nitrates	trace	14.27300
Iron	0.2	13.07300
Fluorine		1.00000
REMARKS:	Water is chemically suitable.	9.00000
		7.00000
		63.40640
		0.08380
		23.00000
		0.00000

*Filtered from 100 to 180 ft  
 Water at 130 ft*

C. Emerson Noble  
 Provincial Analyst

CEN:p1



May 16, 1962

WATER ANALYSIS REPORT  
 CHEMICAL

Submitted by ..... Secretary-Treasurer ..... Date received ..... May 14, 1962 .....  
 Address ..... Town of Vulcan ..... Date reported .....  
 ..... Vulcan, Alberta ..... Source of Sample ..... Vulcan Well # 5 .....  
 Container No. ..... D. 43 ..... Serial No. ....  
 Lab. No. ..... 62 - 4369 .....

PARTS PER MILLION

Total Solids	1910		
Ignition Loss	218		
Hardness	110		110
Sulphates	667		667
Chlorides	64		64
Alkalinity	135		135
Nature of Alkalinity	Bicarbonate of soda, lime and magnesium		20.62265
Nitrites	trace		2.21091
Nitrates	23		16.81510
Iron	0.3		1.80420
Fluorine			1.84295
REMARKS:			90.36387
			9.63910
			73.06870
	Soda - 24.2 grains/gallon. Water is chemically		7.90315
	to high nitrates.		11.83000
			7.19853

*Filtered from 60 to 175 µ  
 Water at 75, 85 & 150 µ.*

*CEN*  
 C. Emerson Noble  
 Provincial Analyst

CEN:pl  
 cc Research Council

**Appendix B**